Instrumentation of Concrete Dams

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Abstract— Instrumentation for dams is mainly required for two purposes, to compare the parameters during actual execution of project with the parameters designed and to detect any unusual or abnormal behavior of structure during construction and post -construction so that any future hazards can be predicted. Instrumentation may differ for different kind of dams. Instrumentation for concrete dams is needed to monitor performance of mainly concrete super structure and foundation. Proper instrumentation plan can evaluate actual behaviour of dam and its pertinent structures. This paper briefly discusses instrumentation of concrete dam along with specific parameters to be monitored, instruments required and frequency of instrumentation for different parameters. A case study of instrumentation is also discussed.

Index Terms— Alkali Aggregate Reaction, Crack Movement, Deformation, Piping.

I. INTRODUCTION

Dams are key structures of any hydro-project as they are designed to withstand enormous pressure created by impounding water. Sudden rise of impounded water pressure can results in loss of lives and property associated with the dam. Therefore, proper safety of dam is extremely important. To keep watch on safe performance of dam, regular monitoring of dam by implementing well planned instrumentation programme is imperative. Instrumentation along with visual observation can play vital role in providing early warning of many conditions that could lead to structure failures and other incidents. Detection of any unusual performance can be helpful in predicting any hazards and for taking remedial measures. Concrete, stone masonry or brick masonry are of heterogeneous type materials, but in designing it is assumed as homogeneous and isotropic. Therefore, states of stress and strain in the structure are different than assumed in design analysis. Proper instrumentation plan of any structure can be of great use in validating the structure's performance with design parameters. For concrete dams, instrumentation is basically done for monitoring the strength and elastic properties of concrete and foundation.

II. CAUSES OF CONCRETE DAMS FAILURES¹

Common factors affecting the performance of concrete dams are structural properties of the concrete and the foundation. These properties may be strength (compressive, tensile, and shear), elastic properties (modulus and Poisson's ratio) and its unit weight. Common causes of failures of concrete dams may be: > Overtopping from inadequate spillway capacity.

➢ Spillway blockage resulting in erosion of the foundation at the toe of the dam

> Foundation leakage and piping in pervious strata.

▶ Rock discontinuities and sliding along discontinuities in foundations.

III. INSTRUMENTATION FOR CONCRETE DAMS

At the outset, it is necessary to appreciate that there is no standard instrumentation plan for any concrete dam. However, as per BIS code IS: 7436 (Part 2):1997² following obligatory and optional measurements are required to be provided for concrete masonry dams.

3.1 Obligatory Measurements

It include the uplift pressure at the base of dam, seepage, temperature (of dam, reservoir water and air), displacement (between two monoliths, between foundation and body of the dam), displacement of any joint of the dam with respect to surrounding set up.

3.2 Optional Measurements

They may be undertaken where warranted by special circumstances of the project, especially provided in high dams, mass concrete structure (MCS) with unusual design or with geological complexities. These measurements are sometimes taken for verification of design criteria, e.g., stress, thermal stress data, thermal response such as strains, pore pressure, seismicity of the area and dynamic characteristics of the structures.

IV. PARAMETERS TO BE MONITORED FOR CONCRETE DAMS

To know about performance of structure, common factors are required to be monitored and analyzed. These common factors may be temperature, water level, flow, precipitation, water quality etc. Apart from these common factors, some specific parameters are required to monitor for concrete dams.

As per ASCE guidelines³, following parameters for concrete dams are generally required to be monitored:

1.1 Concrete Temperature

Temperature changes cause concrete volume changes that affect contraction joints. It also affects growth of expansion in turn cracking in concrete caused by alkali aggregate reaction (AAR). Temperature measurements are important for a proper understanding of behaviour of dam as well as instruments installed on dam throughout the life of dam. Thermocouple, Thermistor an RTD are common devices for measurement of temperature.

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1.2 Movement and Deformation of Dam

Movement can be linear or rotational and usually refer to a fixed point or rigid body while block monolith movement refers to relative movement of the blocks of a concrete dam. Commonly used instruments for measuring movements are plumb line, Inclinometer, Laser plumb line and Tilt meter etc. Deformation strictly refers to change in position/ shape relative to the initial shape at a particular load or stress. Reservoir pressure or change in temperature causes deformation. Deformation can be measured by Normal and Inverted Plumb lines. With advancement in the field of instrumentation, nowadays Electronic Distance Measuring (EDM) instrument is being used for deformation of dam.

1.3 Crack Movement

Cracks are random discontinuities, whereas joints are predefined discontinuities. Almost all concrete structures cracks. There is no standard scheme for selection of instrument locations for measuring crack; however, instruments can be installed according to anticipation and assumptions of crack during design. For crack movement monitoring usually, crack width and/ or relative slip is measured. Crack movement can be measured by Crack meters/ Joint meters installing across the crack.

1.4 Uplift Pressure

Uplift pressure is referred as internal hydrostatic pressure acting on a horizontal section or at its base through the dam. It reduces vertical compressive stresses. Normally, uplift pressure is measured through piezometers or pressure gauge.

1.5 Seepage

Seepage is an important indicator of the performance of all types of concrete dams. The quantity of flow from foundation drains shows the effectiveness of drains in relieving foundation pressure. Normally, seepage is measured through Weir.

1.6 Stress and Strain

Stress monitoring is important in understanding the behaviour of concrete dams and their foundations. Stress can be measured directly by installing Stress meters. Strain can be easily measured by strain gauges. Stresses can also be calculated from strain if mechanical properties (modulus of elasticity and Poisson's ratio) of concrete are known. However, the stress measuring instruments are more expensive and delicate than strain meters and hence, it is a common practice to measure the strain and developed stresses can be calculated from strain.

1.7 Seismic Force

Seismic monitoring of dams generally refers to two kinds of measurements, measurements of strong ground motions with accelerometers and measurement of ground motions with seismograph.

V. INSTRUMENTS RECOMMENDED FOR CONCRETE DAMS

The type, number, range, accuracy etc. of instruments instrument should be installed at such locations that serves the purpose effectively. Figure 1 shows typical concrete gravity dam with location of instruments.



- 1 Measurement of Reservoir Level
- 2 Measurement of strain to determine stress
- 3 Measurement of internal temperature
- 4 Measurement of joint movement
- 5 Plumbline to measure movement
- 6 Fixed tilt meter
- 7 Measurement of uplift pressure at rock-concrete interface
- 8 Foundation Seepage measurement in gallery
- 9 Measurement of tail water

Fig 1: Location of instruments in a Concrete Gravity Dam³

Table 1 shows instruments for monitoring of different parameters for concrete dams as per ASCE guidelines.

Table 1: Concrete dam instruments and measurements (ASC	Е
Guidelines) ³	

Droparty Moogurement Typical Instruments								
Monsurad	Location	i ypicai instruments						
Alignment	Crest or other	Total station lasor						
Anglinent	crest of other	CDS Coodimator						
	interest	GPS, Geodimeter						
Potation	Within concrete	Inclinemator						
Rotation	within concrete	Inclinometer						
	Within foundation	Tiltmeter						
Differential	Across joints or	Strain gauge,						
Movement	cracks	extensometer, joint						
		meter, crack meter.						
	Within foundation	Extensometer, tiltmeter						
Water	Uplift across base	Piezometer, observation						
Pressure	•	well						
	Within concrete	Piezometer						
	At drains	Pressure gauge						
Stress and	Foundation	Total pressure cell, load						
Strain		cell, strain meter, flat						
		jack						
	Within concrete	Total pressure cell.						
		strain meter						
Internal	Within concrete	Thermocouple, RTD,						
Temperature		thermistor.						
Seepage	Any location of	Calibrated container,						
Quantity	interest	weir, flume, flow meter.						
Seepage	Any location of	Turbidity meter						
Quality	interest	-						
Anchor	Anchor head	Load cell, jack and						
Load		pressure gauge.						
Retention								
Earth quake	Crest, free field or	Strong motion						
Response	other surface	accelerometer						
-	location of interest							

VI. FREQUENCY OF MONITORING:

The frequency of instrument observations at a dam depends on several factors including height and age of structure, quantity of water impounded, relative seismic risk. In general, very frequent (even daily) readings should be taken during the first filling of a reservoir, when water levels are high and after significant storms and earthquakes. Immediate readings should be taken following a storm or earthquake. Significant seepage, movement, and stress-strain readings should probably be made at least monthly.

ICOLD, 1988 suggests two important phases of instrumentation as given below⁴:

• Measurements during construction and first filling: Apart of other relevant information, the primary purpose of these measurements is immediate safety.

• Measurements during operation: The main purpose is to offer a reliable picture of all evolutions, some of may be favorable while others may give grounds for concerns.

Visual observations has its own significance and its accuracy depends upon experience of observer. These observations should be made during each visit to the dam and not less than monthly. However, minimum frequency of readings for different types of instrumentation and for different stages is listed in Appendix I [5] for reference.

VII. CASE STUDY

SUPA DAM⁶

Kalinadi Hydroelectric Scheme is a major power project of the country in the western ghats of Karnataka undertaken by the Karnataka Power Corporation, The power is generated through a cascade of dams consisting of Supa dam, Tattihalla Dam, Bommanahalli Dam, Kodasalli Dam and Kadra Dam. The project is situated in the rugged terrain for optimum utilization of power potential. Supa Dam is the main storage dam for all the stages of Kalinadi Hydroelectric Scheme. Supa dam has a total generation capacity of 100 MW with two units of 50 MW each.

It is a 101.00 m high concrete gravity structure founded on complex geological settings such as intricate folding, shearing and faulting, dyke intrusion and deep weathering of rock formation on either abutment. The entire length of Supa Dam is divided into 20 blocks of various widths. Because of the severity of unfavorable foundation conditions and the unconventional design adopted for the block, the structural behaviour parameters such as deformations, deflections, etc. are being monitored by recording and evaluating periodically with the help of intensive instrumentation.

Various types of instruments are embedded in blocks 4, 5, 7 and 11 to monitor the structural parameters such as uplift, stresses, temperature, displacements and pore pressure, etc. (Fig. 2). 23 numbers of uplift pressure cells are embedded in blocks 4, 5 and 7. It is seen from the data collected that the observed uplift in respect of blocks 4, 5 and 7 is within 33% of head of water. 22 numbers of foundation drains have been drilled in blocks 1 to 19 to release uplift from foundation drainage water. No uplift is noticed from the foundation drains of blocks 1 to 4 and blocks 9 to 10.



Fig.2: Instrumentation of Supa dam. (Courtesy: Instrumentation of Dams" CBIP, publication no.287)

VIII. CONCLUSION

Instrumentation of concrete dams is important to monitor the performance of concrete dam and its foundation. Almost any parameter viz. stress, strain, temperature joint openings, foundation movement, pore water pressure etc., which can affect the performance of the dam, can be measured by using proper instrument. This is of utmost importance that we should know, what is to be measured and for what purpose. Obligatory measurements of parameters (viz. temperature, uplift pressure, seepage deformation, movement) should be done for all concrete dams. However, optional measurements (viz. seismic parameters, stress, strain etc.) can be monitored as per specific requirements. Instruments for long term monitoring should be rugged, easy to maintain and should be able to be calibrated regularly. It is also important that threshold values (based on design of dam) for all parameters to be monitored should be evaluated beforehand, so that any abnormal data observed by instrumentation could be noticed and remedial measures can be taken well in time.

REFERENCES

- [1] Chapter IX Instrumentation and Monitoring (pp. 9-2).
- [2] BIS 7436 (Part-II): 1997 "Guide for types of measurements for structures in river valley projects and criteria for choice and location of measuring instruments (part II- concrete and masonry dams)".
- [3] "Guidelines for Instrumentation and Measurements for Monitoring Dam Performance" American Society of Civil Engineers (ASCE). Page nos.2-9, 2-15 and 10-3 to 10-19.
- [4] "Dam Monitoring General Considerations", ICOLD Bulletin No. 60 (1988).

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[5] "Concrete Dam Instrumentation Manual" A Water Resources Technical Publication, United States Department of the Interior, Bureau of Reclamation. Page no.17. [6] "Instrumentation of Dams" CBIP, publication no.287. Page no.318, 329-332.

Appendix-I⁵

Type of Instruments	During Construction		During Periodic Report	of Operation		
-jpe of more unerto	Construction	Shutdown	Initial Filling	Frist Year	2 to 3 Years	Regular
Vibrating-wire piezometers	Weekly ²	Monthly	Weekly	Biweekly	Monthly	Monthly
Hydrostatic uplift pressure	Weekly	Monthly	Weekly	Weekly	Biweekly	Monthly
pipes			j		J	
Porous-tube piezometers	Monthly	Monthly	Weekly	Weekly	Monthly	Monthly
Slotted-pipe piezometers	Weekly	Monthly	Weekly	Weekly	Biweekly	Monthly
Observation wells	Weekly	Monthly	Weekly	Weekly	Biweekly	Monthly
Water levels	Weekly	Monthly	Weekly	Weekly	Monthly	Monthly
Seepage measurement	-	-	Weekly	Weekly	Biweekly	Monthly
(weirs, flumes, etc.)						
Visual seepage monitoring	Weekly	Weekly	Weekly	Weekly	Biweekly	Monthly
Resistance thermometers	Twice weekly	Monthly	Weekly	Weekly	Monthly	Monthly
Thermocouples	Daily	Monthly	Weekly	Weekly	Monthly	Monthly
Carlson strain meters	Weekly	Weekly	Weekly	Biweekly	Monthly	Monthly ³
Joint meters	Weekly	Weekly	Weekly	Biweekly	Monthly	Monthly ³
Stress meters	Weekly	Monthly	Weekly	Biweekly	Monthly	Monthly ³
Reinforcement meters	Weekly	Monthly	Monthly	Monthly	Monthly	Monthly ³
Penstock meters	Weekly	Monthly	Monthly	Monthly	Monthly	Monthly ³
Deflectometers	Weekly	Monthly	Weekly	Weekly	Monthly	Monthly
Vibrating- wire strain	Weekly	Monthly	Monthly	Monthly	Monthly	Monthly
gauge	•	-	-	-	-	•
Vibrating-wire total pressure cell	Weekly	Monthly	Monthly	Monthly	Monthly	Monthly
Load cell	Weekly	Monthly	Weekly	Biweekly	Monthly	Monthly
Pore pressure meters	Weekly	Weekly	Weekly	Biweekly	Monthly	Monthly
No-stress strain meters	Weekly	Weekly	Weekly	Biweekly	Monthly	Monthly
Foundation deformation meters	Weekly	Weekly	Weekly	Biweekly	Monthly	Monthly
Flat jacks	Daily	Weekly	Weekly	Biweekly	Monthly	Monthly
Tape gauges(tunnel)	Weekly	Weekly	Weekly	Biweekly	Monthly	Monthly
Whittemore gauges	-	-	Biweekly	Biweekly	Monthly	Monthly
Avongard crack monitor	Weekly	Monthly	Weekly	Weekly	Monthly	Monthly
Wire gauges	-	-	Monthly	Monthly	Monthly	Quarterly
Abutment deformation gauge	Weekly	Monthly	Weekly	Weekly	Monthly	Monthly
Ames dial meters	Weekly	Monthly	Weekly	Weekly	Monthly	Monthly
Differential buttress	Weekly	Monthly	Weekly	Weekly	Monthly	Monthly
gauges						
Plumblines	Daily	Weekly	Daily	Weekly	Biweekly	Monthly
Inclinometer	Weekly	Weekly	Weekly	Weekly	Biweekly	Monthly
Collimation	Every other day for a month	Monthly	Weekly	Biweekly	Monthly	Monthly
Embankment settlement	-	-	Monthly	Bimonthly or six times	Quarterly	Two times
points				per year		per year
Level points	Monthly	Quarterly	Monthly	Monthly	Six times per	Six times pe
					year	year
Multipoint extensometers	Weekly	Monthly	Weekly	Monthly	Monthly	Two times per year
Triangulation	-	-	Monthly	Monthly	Quarterly	Quarterly
Trilateration(EDM)	-	-	Biweekly	Monthly	Quarterly	- •
Reservoir slide monitoring	-	-	Monthly	Monthly	Monthly	Quarterly
systems			5		2	
Power plant movement	-	-	Weekly	Monthly	Monthly	Monthly
Pool movement	Weekly	Monthly	Weekly	Monthly	Monthly	Monthly

¹These are suggested minimum; however, anomalies or unusual occurrences such as earthquakes or flood will require additional readings. ²Daily during curtain grouting.

³May be discontinued after 3 years unless anomalies are noted.

(Ref: Concrete Dam Instrumentation Manual; A water resources Publication, United States Department of the Interior, Bureau of Reclamation, October, 1987, Table 1-3, Page no.9